



88653

INVESTIGATION
OF
FUEL OIL LEAK
FOR
HEXCEL
LODI, NEW JERSEY

BY
TENECH ENVIRONMENTAL ENGINEERS, INC.

I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT
WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT
I AM A DULY REGISTERED PROFESSIONAL ENGINEER UNDER THE LAWS
OF THE STATE OF NEW JERSEY.

REGISTRATION NO. GE 25662

6/27/84

Date

Mark W. Tenney
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Purpose

The Hexcel Corporation plant located in Lodi, New Jersey has experienced a fuel oil leak from a buried storage tank. An investigation was begun in May 1984. The purpose of this report is to present the findings of this investigation. Included in this report are boring logs, laboratory testing program descriptions and results, soil descriptions, conclusions and recommendations.

Subsurface Exploration Borings

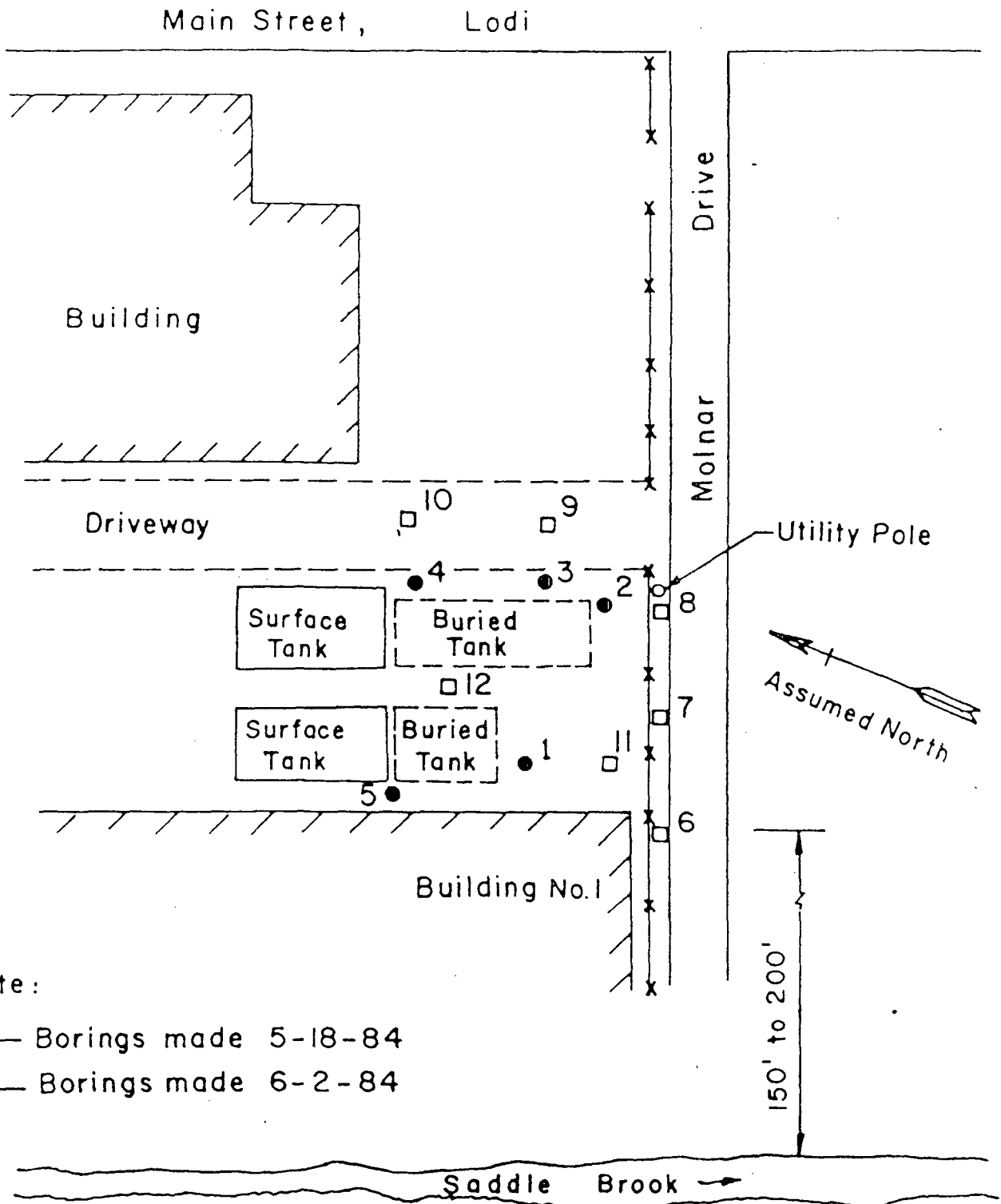
In order to evaluate the lateral and vertical extent of migration of the fuel oil away from the buried tanks, twelve borings were made during May and June of 1984. Locations of the borings are shown on the attached map (Figure No. 1). It would have been desirable to have completely ringed the tanks with borings, but the existing structures, roadways and property boundaries limited possible drill sites. Areas north and west of the tanks could not be accessed.

Drilling to obtain samples was performed by the Warren George Company. All borings were accomplished using a Spreague and Henwood truck-mounted drill rig. Continuous soil samples were obtained utilizing a standard penetration sampler driven with a 140-pound hammer. The sampler was driven for a distance of two feet, then withdrawn and the sample removed. Blow counts for each six-inch increment of depth were recorded in the logs. Materials removed from the sampler were visually inspected and described in the field for purposes of the drill logs. Samples of the materials for laboratory testing were obtained for each depth interval sampled. Material for chemical testing was placed in glass jars and that for physical testing was placed in plastic sample bags.

On May 19, 1984 five borings (B1 through B5) were made in the vicinity of the tanks. Most of the borings were in very close proximity to the tanks, (i.e., within one or two feet of the tanks). Locations of the borings are shown on the attached location map (Figure No. 1). Borings were made on all but the north side of the tanks. Borings B4 and B5 are located on the east and west sides at the north end. Boring logs for the five holes are attached.

On June 2, 1984 seven new borings (B6 through B12) were made along the east and south sides of the tanks. These borings were located approximately ten additional feet away from the tanks. The locations of these new borings can be described as comprising half of a second larger diameter concentric ring around the tanks.

BORING LOCATION MAP



Note:

- — Borings made 5-18-84
- — Borings made 6-2-84



Environmental Engineers, Inc., South Bend, Indiana

HEXCEL, Lodi

Scale	None	Dr	hpa	Ckd
	6-8-84	Figure:	1	



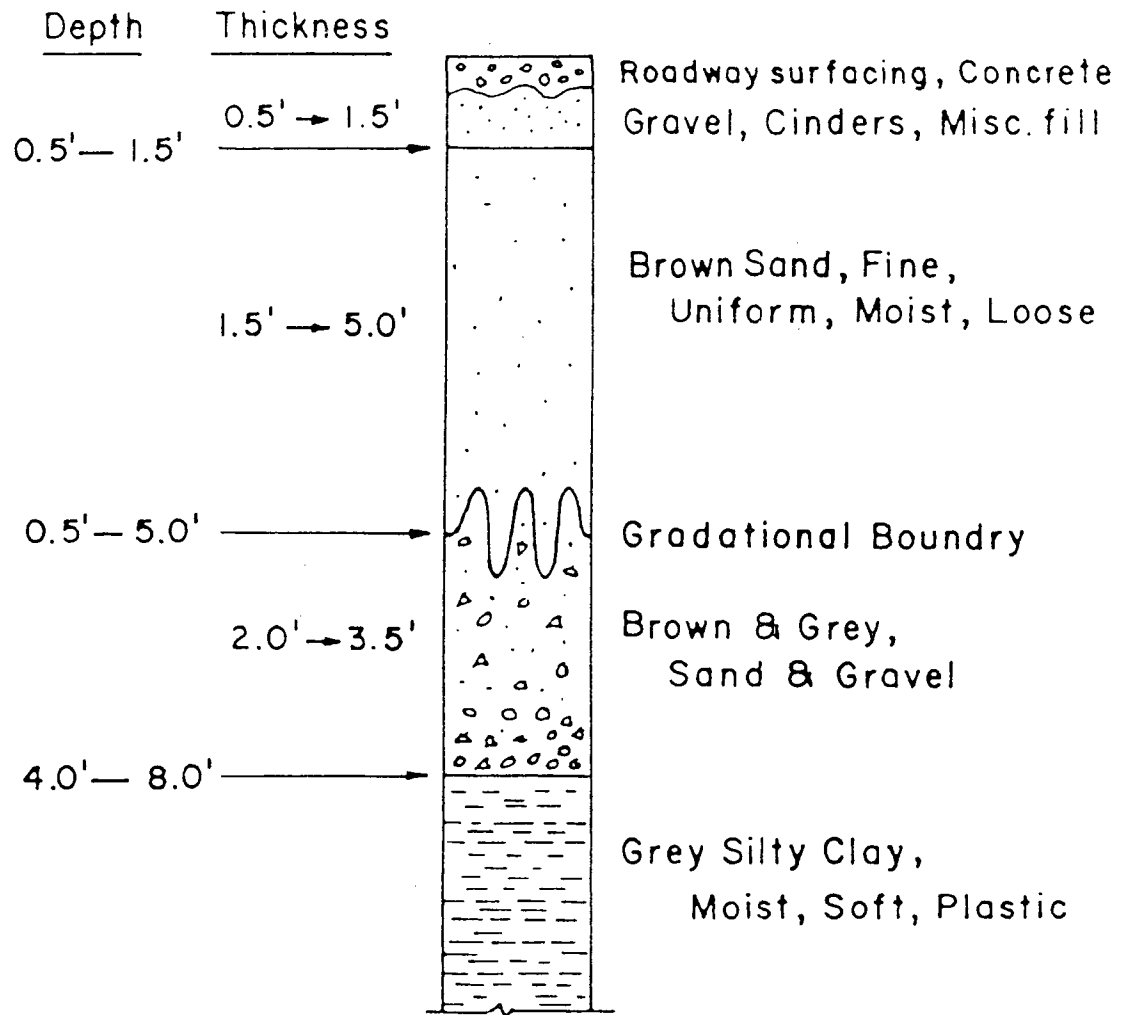
Soil Profile

The twelve borings performed identified three relatively consistent soil layers in the area of the tanks. A typical soil profile has been developed from the descriptions in the drill logs. Figure No. 2 is a graphical presentation of that profile. Each of the soil layers can be described as follows:

1. Roadway Surfacing And Concrete The entire area overlaying the tanks is covered by either concrete or roadway surfacing material. Underlying the surfacing is either gravel or cinders. The combined thickness of these materials ranges from 0.5 to 1.5 feet and averages approximately 1.0 feet in thickness.
2. Soil Layer #1, Brown Sand The first soil layer consists of brown, relatively uniform sand and silt. This sand is moist and firm, with an average N value of approximately 10. The density of the sand is not uniform as N values ranged from 2 to 28. Thickness of this layer ranges from 1.5 to 5.0 feet.
3. Soil Layer #2, Grey Sand And Gravel Underlying the brown sand is a grey sand and gravel layer. The size and percentage of gravel appears to increase with depth. It is highly probable that the boundary between the brown and grey sand is gradational. Thickness of this layer ranges from 2.0 to 3.5 feet. An average N value for this layer is 10 with values ranging from 1 to 22.
4. Soil Layer #3, Grey Silty Clay Consistently and uniformly underlying the entire area is a silty clay layer. This layer is grey, green and brown in color. Its consistency is moist and firm. Thickness of this layer is consistently greater than 1.5 feet. N values in the layer ranged from 3 to 22, with an average value of 10. Some naturally occurring organic materials are present in this layer.

There is a high probability that the upper part of the brown sand layer has been disturbed by past construction activities at the site. The potential also exists that it is a fill material, which had been brought in to level the plant site. It also appears to have been used as the backfill material around the tanks.

TYPICAL SOIL PROFILE



TenEch Environmental Engineers, Inc., South Bend, Indiana

HEXCEL, Lodi

Scale —	Dr hpa	Ckd
6-8-84	Figure:	2



Physical Testing of Soils

Samples for physical testing of each soil layer were collected from Borings B9, B10, and B11. Material from each boring was composited into one bagged sample for each layer. It was necessary to composite material to obtain an adequate volume of material for testing. Laboratory tests performed on each sample consisted of a gradation analyses performed combining both sieves and hydrometer. Atterberg limits were performed to determine the moisture content-consistency relationship for the soils and to allow the soils to be classified using the Unified Soil Classification System. Laboratory testing was performed by Stokley-Cheeks and Associates of Lexington, Kentucky. The laboratory work sheets and results are appended to this report. Laboratory procedures used were Standard ASTM Methods. Methods used were D422 for grain size, D423 for liquid limit, D424 for plastic limit and D2216 for moisture contents.

Table 1 presents a summary of the laboratory test results and Unified Soil Classification for each soil layer. It can be seen in the table that layers #1 and #2 are predominantly sand with significant percentages of both silt and gravel being present. Results also indicate that the soil layer #2 has approximately 20 percent more gravel and sand size particles than soil layer #1. These results indicate a coarsening downward within layers #1 and #2. The Unified Soil Classification for layer #1 of SM indicates that the layer consists of both silt and sand size particles. The classification of SP-SM for layer #2 indicates that gravel, sand and silt size particles comprise the material. Atterberg limits for both layers indicates that the soils are essentially non plastic.

Layer #3 is significantly different from the overlaying sands. It consists of 95 percent silt and clay size particles. The classification CL-ML for the soil indicates that it is a silty clay of low plasticity. Naturally occurring organic materials are present in this layer, but comprise a very small percentage.

Based on the Unified Soil Classifications for the two sand layers, the permeability of the layers could be expected to be within the range of 10^{-3} to 10^{-5} feet/minute. The presence of silt and clay size particles in these layers, depths from 0.5 to 8.0 feet, significantly reduces the potential permeability of these layers. The coarsening downward within the sand layers would result in an increasing permeability with depth. Permeability of the silty clay layer, located at a depth of 8.0 feet and below, could be expected to be on the order of 10^{-7} feet/minute. This layer is essentially impermeable. The sand layers can be considered to be moderately permeable. The values of permeability were taken from tables of typical values in Design of Small Dams, by the Bureau of Reclamation and Soil Mechanics, Foundations and Earth Structures Navfac DM 7, by the Naval Facilities Design Command.

TABLE 1

PHYSICAL TEST RESULTS FOR SOIL

LAYER DEPTH	#1 0.5 - 5.0	#2 4.0 - 8.0	#3 8.0 & BELOW
GRAVEL	4%	15%	0%
SAND	58%	76%	5%
SILT	31%	7%	75%
CLAY	7%	2%	20%
LIQUID LIMIT	26	28	21
PLASTIC LIMIT	25	27	17
PLASTIC INDEX	1	1	4
UNIFIED SOIL CLASSIFICATION	SM	SP-SM	CL-ML



Occurance of Oil

The objective of the exploration program was to locate and determine the extent of movement of fuel oil in the ground surrounding the tanks. For the twelve borings made, signs of oil were visually observed in nine of the borings. There was no oil visually observed in borings B3, B5 and B6. At a later date oil was observed on the water in B3.

The amount of oil occurring in the samples varied between the borings. For borings in close proximity to the tanks, oil could visually be seen in the samples. For the outer ring of borings, the amount of oil observed was less. This fact was established based on the driller's observations and comments during personal discussions with him. The presence of oil consisted of a sheen on the sample or on water in the sampler. It is also substantiated by visual inspection and comparisons of the samples.

The occurrence of oil was principally within the grey sand and gravel layer. Within that layer, the oil appeared to be greater near the bottom. This would tend to be expected as the layer is coarser toward the bottom and permeability is higher. Some trace amounts of oil were observed within the brown sand layer.

Significant conclusions which can be drawn follow:

- Oil has leaked from the tank and is principally present in the grey sand and gravel layer, and backfill surrounding the tank.
- The amount of oil present in the decreases with distance from the tank.
- The permeability of the sand and gravel layers, depths ranging from 0.5 to 8.0 feet, is moderate and the oil could have moved some limited distance through the layers.



Oil Content Testing

For the purposes of determining oil content of the soil surrounding the tanks, samples were tested from three borings. Samples covering the entire thickness of layers #1, and #2 and the top of layer #3 were tested from borings B6, B8 and B10. The oil and grease content was determined using the standard method given in "Procedures For Handling and Chemical Analysis of Sediment and Water Samples," (EPA/CE-81-1), May, 1981. The exact method used was; Oil and Grease Method 1, Freon Extraction, page 3-284. This method reports the free and extractable oil and grease content of a material on a dry weight basis. Results are reported in units of mg/Kg or parts per million. Laboratory testing was conducted by Aqualab, Inc. of Streamwood, Illinois. The laboratory report is appended to this report.

Table 2 presents a summary of the laboratory test results for the oil and grease content testing. Reported in the table is the oil content in parts per million and also in percentage by dry weight. Within the sand layers, the oil contents range from 1,250 to 10,100 parts per million. Within the silty-clay layers, the oil contents range from 149 to 650 parts per million. On review of the results in the table, it can be seen that the last sample in each boring has a significantly lower oil and grease content than those at shallower depths. The last sample in each boring is from the third soil layer, the silty-clay. Within the sand layers the oil content averaged 0.48 percent. Within the silty clay layer, the oil content averaged 0.03 percent. It is highly probable that the 0.03 percent value is a natural background amount due to the naturally occurring organics present in the soil.

Based upon a review of the New Jersey Hazardous Waste Regulations, it would appear that none of the soil samples tested would be representative of a hazardous waste. The New Jersey regulations (Revision 84-1) define New Jersey Hazardous Waste Number X725 as follows:

Oil spill cleanup residue which: A) is contaminated beyond saturation; or B) the generator fails to demonstrate that the spilled material was not one of the listed hazardous waste oils, (see page 8-14a of the New Jersey Hazardous Waste Regulations).

Fuel oil, when stored and used for its original purpose, is not a hazardous waste under the regulations, so only Part A must be addressed. The soil samples collected at the Lodi facility all contained 1.01 percent oil or less. None of the samples were found to be saturated with oil and, therefore, should not be considered hazardous wastes.

TABLE 2

SAMPLE OIL CONTENT

BORING NO.	DEPTH (FT)	SAMPLE NO.	OIL CONTENT PPM (Mg/Kg)	PERCENT OIL CONTENT
B6	1.0 - 3.0	8000	1250	0.13%
	3.0 - 5.0	8001	3880	0.39%
	5.0 - 7.0	8002	3610	0.36%
	7.0 - 8.0	8003	4000	0.40%
	9.5 -10.5	8004	149	0.01%
B8	2.5 - 5.5	8009	7950	0.80%
	5.5 - 7.0	8010	6070	0.61%
	8.0 - 9.0	8011	650	0.07%
B10	3.0 - 5.0	8013	10100	1.01%
	5.0 - 7.0	8014	3890	0.39%
	7.0 - 8.0	8015	2280	0.23%
	9.5 -11.0	8016	174	0.02%



Ground Water Levels

The occurrence of ground water was noted in all the borings. Water levels in all the borings was measured on June 2, 1984. The depth below the existing ground surface to the water level in the holes ranged from 4.0 to 4.5 feet. Several of the holes had caved in at that depth and a wet sand slurry was present below that depth.

It should be noted that ground water levels may have been abnormally high at the time the measurements were made due to the unusually heavy rains which had occurred.

Factors controlling ground water movement and ground water levels in the area are as follows:

- Saddle Brook river located approximately 150 to 200 feet west of the tanks is the hydrostatic low in the immediate area of the tanks. Flow of ground water in the sand layers could be expected to be toward the river.
- The location and elevation of the silty clay layer has perched the water table and prevented vertical movement of water and oil through the layer.
- The permeability of the sand layer is moderate, 10^{-3} to 10^{-5} feet/minute. Estimating that the Saddle Brook river is located 5 feet lower in elevation than the water table in the borings, the maximum flow velocity toward the river would be 0.05 feet per day based on the following calculation (with an assumed permeability of 10^{-3} feet/minute:

$$V = ki$$

$$V = (1 \times 10^{-3} \text{ ft/min}) \frac{5 \text{ ft}}{150 \text{ ft}} \times 1440 \frac{\text{min}}{\text{day}}$$

$$V = 0.05 \text{ ft/day}$$

If the permeability is 10^{-5} feet/minute, the flow rate would be 0.0005 feet/day. These calculations indicate that the movement of ground water toward the Saddle Brook river is at a slow to moderate rate.



Conclusions

Conclusions which can be drawn based on the information which is available are as follow:

- Fuel oil is present in the sandy soils immediately surrounding the buried tanks. The vertical extent of oil contaminated soil is limited because of the presence of the silty clay layer.
- The amount of oil present in the sand and gravel layers, depths ranging from 0.5 to 8.0 feet, decreases with distance from the tanks. This fact is supported by both visual observations and laboratory test results. The moderate permeabilities of the sands has limited the lateral migration of the oil.
- The silty clay layer has not been contaminated by the fuel oil. Visual observations and laboratory test results support this fact.
- Soils located at a distance of 10 feet or greater from the tanks have oil contents equal to or less than 1.01 percent based on their dry weight.
- Thickness of the sand and gravel layer ranges from four to eight feet. Flow of water in this layer would be expected to be toward the Saddle Brook river, located to the south-west of the buried tanks. If additional sampling or drilling is desired, to check the downgradient conditions in more detail, this could be conducted in the vicinity of borings Number 1 and 11.

STOKLEY-CHEEKS AND ASSOCIATES, INC.
GEOTECHNICAL ENGINEERS & GEOLOGISTS

ESTABLISHED 1956

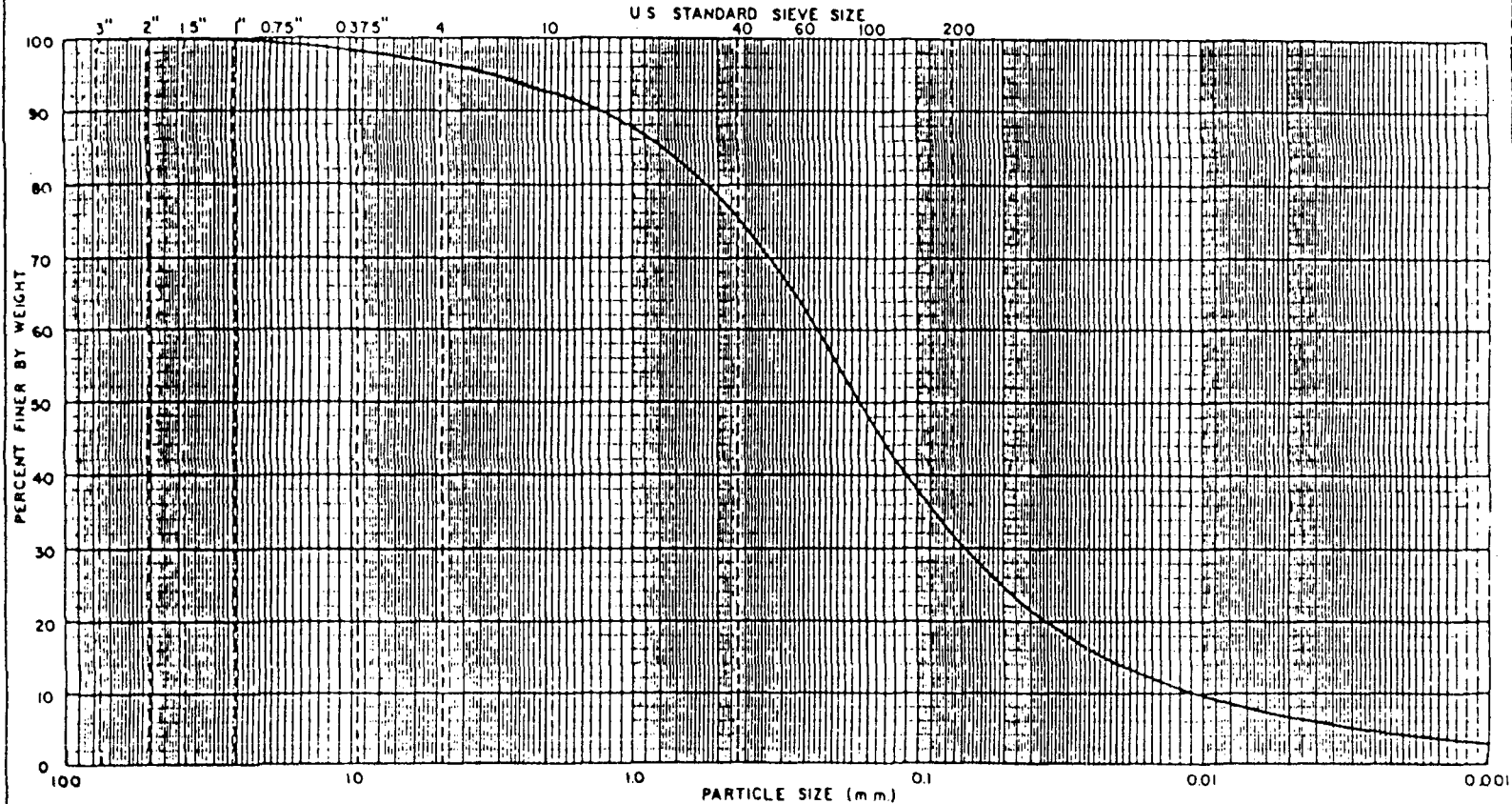
2520 REGENCY ROAD, SUITE 106, LEXINGTON, KENTUCKY 40503

TELEPHONE (606) 278-3402

SUMMARY OF LABORATORY TEST DATA																							
BORING NO.	SAMPLE NO.	DEPTH, FEET	SAMPLE TYPE	SOIL CLASSIFICATION	NATURAL WATER CONTENT, %	WET UNIT WT. ,PCF	DRY UNIT WT. ,PCF	SPECIFIC GRAVITY	GRAIN SIZE					LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	UNCONFINED STRENGTH, KSF	φ, DEGREES	COHESION, PSF	MAX. DRY DENSITY	OPTIMUM MOISTURE CONTENT, %	COMPRESSION INDEX	CBR
									GRAVEL, %	SAND, %	FINES (SILT & CLAY) %	SILT, %	CLAY, %										
Bag	1	Bag	Bag	SM				2.66 Assumed	4	58	38	31	7	26	25	1							
Bag	2	Bag	Bag	SP-SM				2.66 Assumed	15	76	9	7	2	28	27	1							
Bag	3	Bag	Bag	CL-ML				2.68 Assumed	0	5	95	75	20	21	17	4							
SC-682									Ten-Ech Testing									SHEET 1 OF 1					

Bler-201

PARTICLE - SIZE ANALYSIS OF SOILS (ASTM D-422)

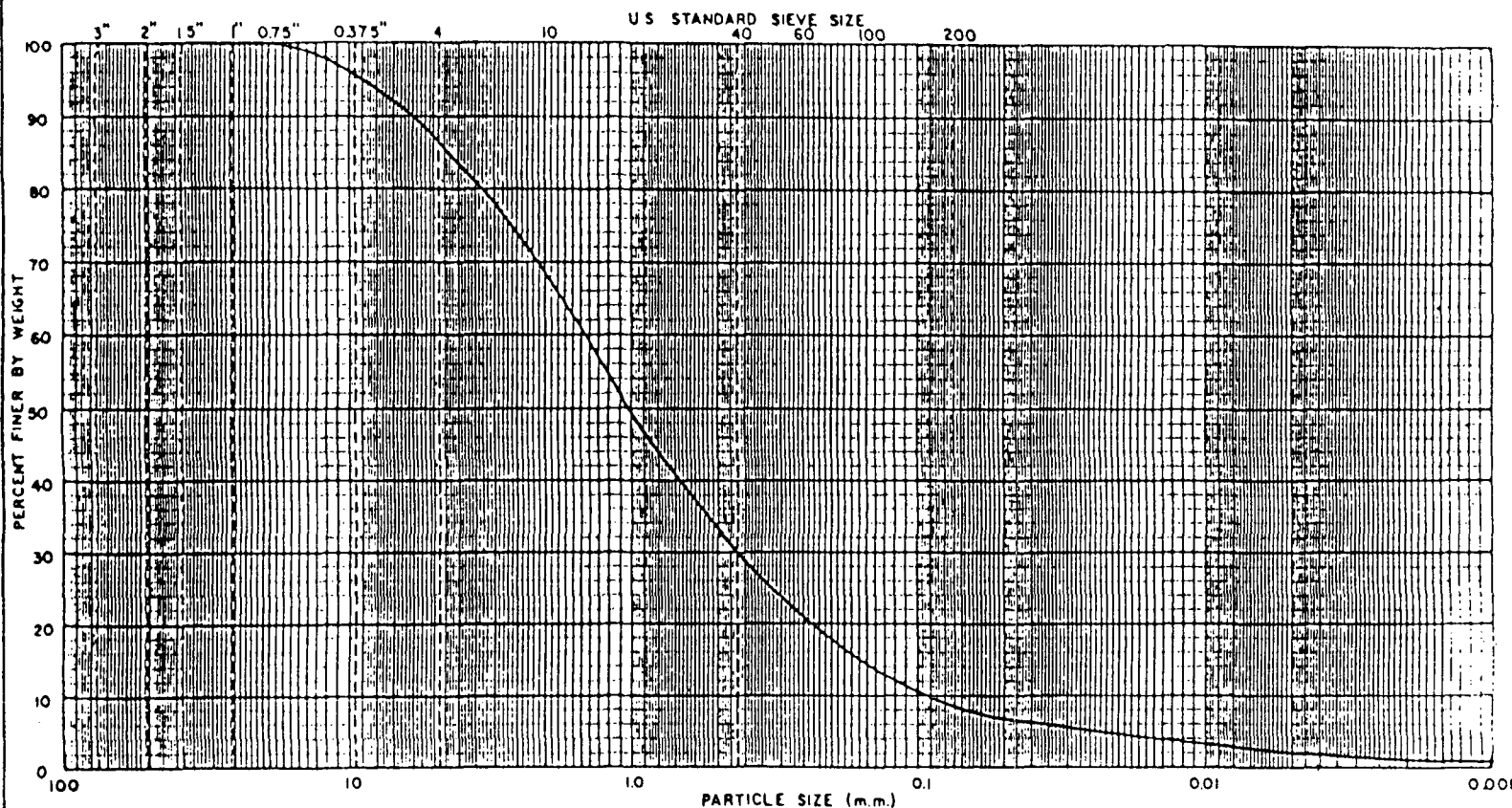


COBBLES (+3") 0 %
 GRAVEL (-3, +NO. 4) 4 %
 SAND (-NO. 4, +NO. 200) 58 %
 a. COARSE SAND (-NO. 4, +NO. 10) 3 %
 b. MEDIUM SAND (-NO. 10, +NO. 40) 13 %
 c. FINE SAND (-NO. 40, +NO. 200) 42 %
 SILT (-NO. 200, +.005 m.m.) 31 %
 CLAY (-.005 m.m.) 7 %
 COLLOIDS (-.001 m.m.) 3 %

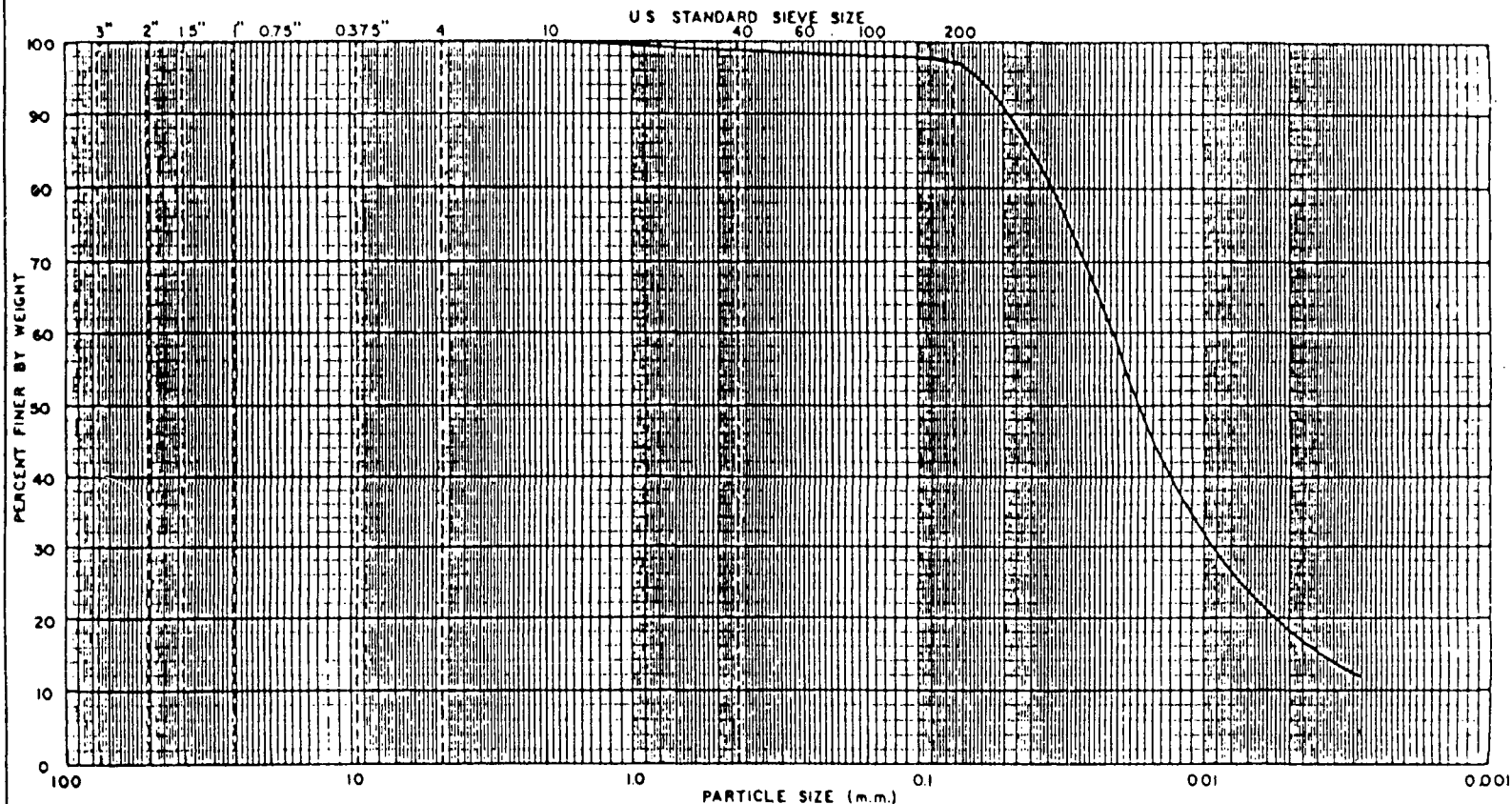
MAXIMUM SIZE OF PARTICLES 3/4"
 DESCRIPTION OF SAND & GRAVEL N/A
 SHAPE (ROUNDED OR ANGULAR) N/A
 HARDNESS N/A
 SPECIFIC GRAVITY 2.66 Assumed
 REMARKS _____

JOB NAME Ten-Ech
 JOB LOCATION N/A
 JOB NO. SC-682
 SAMPLE DATA: BORING NO. Bag
 SAMPLE NO. 1
 DEPTH (FT.) -
 SOIL DESCRIPTION Light Brown, Silty
 Sand
 STOKLEY-CHEEKS & ASSOCIATES, INC.

PARTICLE - SIZE ANALYSIS OF SOILS (ASTM D-422)



U S STANDARD SIEVE SIZE



COBBLES (+3")	0 %
GRAVEL (-3", +NO. 4)	0 %
SAND (-NO. 4, +NO. 200)	5 %
a. COARSE SAND (-NO. 4, +NO. 10)	0 %
b. MEDIUM SAND (-NO. 10, +NO. 40)	1 %
c. FINE SAND (-NO. 40, +NO. 200)	4 %
SILT (-NO. 200, +.005 m.m.)	75 %
CLAY (-.005 m.m.)	20 %
COLLOIDS (-.001 m.m.)	6 %

MAXIMUM SIZE OF PARTICLES #4
DESCRIPTION OF SAND & GRAVEL N/A
SHAPE (ROUNDED OR ANGULAR) N/A
HARDNESS N/A
SPECIFIC GRAVITY 2.68 Assumed
REMARKS

JOB NAME Ten-Ech
JOB LOCATION N/A
JOB NO. SC-682
SAMPLE DATA: BORING NO. Bag
SAMPLE NO. 3
DEPTH (FT.) -
SOIL DESCRIPTION Light Brown Silt
and Lean Clay

STOKLEY-CHEEKS & ASSOCIATES, INC.

CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES

ASTM Designation: D 2487 - 83
(Based on Unified Soil Classification System)

SOIL ENGINEERING

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests ^a				Soil Classification		
				Group Symbol	Group Name ^b	
Coarse-Grained Soils More than 50% retained on No. 200 sieve	Gravels More than 50% coarse fraction retained on No. 4 sieve	Clean Gravels Less than 5% fines ^c	$C_u \geq 4$ and $1 \leq C_c \leq 3^d$	GW	Well graded gravel ^e	
			$C_u = 4$ and/or $1 > C_c > 3^d$	GP	Poorly graded gravel ^e	
		Gravels with Fines More than 12% fines ^c	Fines classify as ML or MH	GM	Silty gravel ^{f, g}	
			Fines classify as CL or CH	GC	Clayey gravel ^{f, g}	
	Sands 50% or more of coarse fraction passes No. 4 sieve	Clean Sands Less than 5% fines ^c	$C_u \geq 6$ and $1 \leq C_c \leq 3^d$	SW	Well-graded sand ^e	
			$C_u = 6$ and/or $1 > C_c > 3^d$	SP	Poorly graded sand ^e	
		Sands with Fines More than 12% fines ^c	Fines classify as ML or MH	SM	Silty sand ^{f, g}	
			Fines classify as CL or CH	SC	Clayey sand ^{f, g}	
Fine-Grained Soils 50% or more passes the No. 200 sieve	Sils and Clays Liquid limit less than 50	inorganic	$PI > 7$ and plots on or above "A" line ^f	CL	Lean clay ^{h, i}	
			$PI < 4$ or plots below "A" line ^f	ML	Silt ^{h, i}	
		organic	Liquid limit - oven dried < 0.75 Liquid limit - not dried	OL	Organic clay ^{h, i, j} Organic silt ^{h, i, j}	
		Sils and Clays Liquid limit 50 or more	inorganic	PI plots on or above "A" line	CH	Fat clay ^{h, i}
				PI plots below "A" line	MH	Elastic silt ^{h, i}
	organic		Liquid limit - oven dried < 0.75 Liquid limit - not dried	OH	Organic clay ^{h, i, j} Organic silt ^{h, i, j}	
	Highly organic soils	Primarily organic matter, dark in color, and organic odor			PT	Peat

^aBased on the material passing the 3-in. (75-mm) sieve

^bIf test sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name

^cGravels with 5 to 12% fines require dual symbols

GW-GM well-graded gravel with silt

GW-GC well-graded gravel with clay

GP-GP poorly graded gravel with silt

GP-GC poorly graded gravel with clay

^dSands with 5 to 12% fines require dual symbols

SW-SM well-graded sand with silt

SW-SC well-graded sand with clay

SP-SM poorly graded sand with silt

SP-SC poorly graded sand with clay

$$C_u = \frac{D_{60}}{D_{10}} \quad C_c = \frac{(D_{30})^2}{D_{10} D_{60}}$$

^eIf and contains $\geq 15\%$ sand, add "with sand" to group name

^fIf fines classify as CL, ML, use dual symbol GC, GM, or SC, SM

^gIf fines are organic, add "with organic fines" to group name

^hIf soil contains $\geq 15\%$ gravel, add "with gravel" to group name

ⁱAtterberg limits plot in hatched area, soil is a CL, ML, silty clay

^jIf soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant

^kIf soil contains $\geq 30\%$ plus No. 200, predominantly sand, add "sandy" to group name

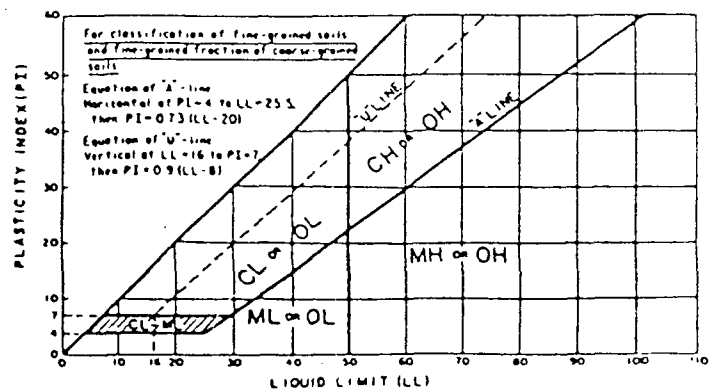
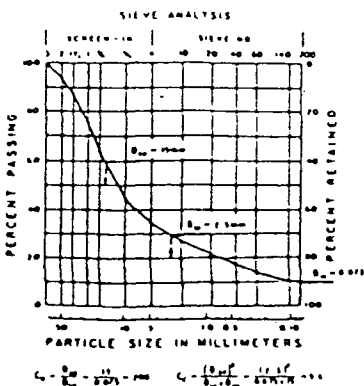
^lIf soil contains $\geq 30\%$ plus No. 200, predominantly gravel, add "gravelly" to group name

^m $P_L > 4$ and plots on or above "A" line

ⁿPlots on or below "A" line

^o P_L plots on or above "A" line

^p P_L plots below "A" line



JOB LOCATION 205 Main St. Loc. N.		WARREN GEORGE, INC. FOOT OF JERSEY AVENUE P. O. BOX 413 JERSEY CITY, N.J. 07303 FOR: <u>Hexcel Corporation</u>		SHEET <u>1</u> OF <u>1</u> LOCATION <u>Main St. Loc.</u> HOLE NO. <u>1-1</u> LINE & STA. _____ OFFSET _____			
DEPTH <u>54'</u> FT. FT. CASING OUT DATE: _____ DATE, START: <u>5/18/84</u>		GROUND ELEVATION _____		DEPTH _____ FT. ALL CASING OUT DATE: _____ DATE, FINISH: _____		GROUND WATER ELEVATION _____	
CASING O.D. _____ I.D. _____ SAMPLER O.D. _____ I.D. _____ DIAMOND BIT SIZE <u>2</u>		WEIGHT OF HAMMER <u>300-140</u> LBS. INSIDE LENGTH OF SAMPLER _____ IN.		HAMMER FALL _____ CASING <u>24</u> SAMPLER <u>30</u>			

DEPTH FEET	CASING BLOWS PER FOOT	SAMPLE NUMBER	SAMPLE DEPTH ELEV. FEET	SAMPLE REMARKS	BLOWS PER FT. ON SAMPLER	DENSITY OR CONSIST MOISTURE	PROFILE CHANGE DEPTH	FIELD IDENTIFICATION OF SOILS REMARKS
0								3'6" Concrete 6'4" Linders Fill
1			0-2'		1-2-7-3			
2			2-4'		3-3-1-2			
3			4-6'		1-0-2-5			1'5" Brown Fine Silty Sand
4			6-8'		A 10-5-4-5			5'6" Gray Brown Medium Coarse Sand Medium Gravel - Gills
5			8-10'		10-5-4-5 1-1-2-4			10'1" Brown Fine Silty Sand Little cl
10								
20								
30								
40								

Soils Engineer: _____	Driller: <u>George Rutschera</u>
Drilling Inspector: _____	Helper: <u>Frank Jones</u>

TEST BORING RECORD

Driller Warren George, Inc (G. Kutschera) Project No. Hexcel Corporation, Lodi
 Ground Water Elev. _____ Consultant _____
 Date Started 6-2-84 Date Completed 6-2-84 Station No. _____
 Size Casing _____ Total _____ ft. Hole No. B6 Surface Elev. _____
 Sample Size _____ Sheet 1 of _____ Sheets
 Total No. Core Boxes _____ Sample Hammer Wt. 140 # Lbs.: Drop 30 Ins.
 Size Core _____ Size Auger _____

ELEVATION	DEPTH	CLASSIFICATION OF MATERIAL	BLOWS & TIME	SAMPLE NO. & RUN	CORE RECOVERY	CORE LOSS	REMARKS
	1.0	Road surfacing and gravel		7999			
	3.0	Redish Brown Sand Moist, Firm	9/9/9/7	8000			
	5.0		5/9/10/21	8001			4.0' Water Level
	6.0		14/12/10/9	8002			
	7.0	Redish Brown And Grey Sand, Wet, Course Sand and Gravel Near Bottom of Layer					
	8.0	Grey Silty Clay Varved, Firm, Moist	8/10/10/9	8003			← Possible Show of Oil, Odor
	9.0	Plastic	7/9/13/14	8004			
	11.0	Stopped Hole 11.0'					

TEST BORING RECORD

Driller Warren George, Inc. (G. Kutschera)

Project No. Hexcel Corporation, Lodi

Ground Water Elev. _____

Consultant _____

Date Started 6-2-84 Date Completed 6-2-84

Station No. _____

Size Casing _____ Total _____ ft.

Hole No. B7 Surface Elev. _____

Sample Size _____


Sheet 1 of 1 Sheets

Total No. Core Boxes _____

Sample Hammer Wt. 140 # Lbs. Drop 30" Ins

Size Core _____

Size Auger _____

ELEVATION	DEPTH	CLASSIFICATION OF MATERIAL	BLOWS & TIME	SAMPLE NO. & RUN	CORE RECOVERY	CORE LOSS	REMARKS
		Road Surfacing, Gravel Cinders, Fill, Misc.					Sample Not Taken
	1.5	Brown Silty Sand Uniform, Loose, Moist	6/5/5/2	8005			
	3.5						
		 Redish Brown And Grey Sand, Coarse	1/1/2/1	8006			4.2' Water Level
	5.5	sand And Gravel At Bottom of Layer					} Show of Oil
	7.0	Wet, Loose	2/3/5/6	8007			
	7.5	Grey And Green Silty Clay Varved, Moist, Plastic					
			1/2/5/6	8008			
	9.5	Stopped Hole 9.5'					

TEST BORING RECORD

Driller Warren George, Inc. (G. Kutchera)

Project No. Hexcel Corporation, Lodi

Ground Water Elev. _____

Consultant _____

Date Started 6-2-84 Date Completed 6-2-84

Station No. _____

Size Casing _____ Total _____ ft.

Hole No. B8 Surface Elev. _____

Sample Size _____

Sheet 1 of 1 Sheets

Total No. Core Boxes _____

Sample Hammer Wt. 140# Lbs. Drop 30" Ins.

Size Core _____

Size Auger _____

ELEVATION	DEPTH	CLASSIFICATION OF MATERIAL	BLOWS & TIME	SAMPLE NO & RUN	CORE RECOVERY	CORE LOSS	REMARKS
	1.5	Roadway Surfacing, Gravel, Cinders, Fill Very Firm	18/19/7				Sample Not Taken.
	3.5		5/7/9/11				No Sample Recovered
	5.5	Brown, Grey & Black Sand and Gravel Loose, Wet	5/5/4/4	8009			Odor of Oil 4.5' Water Level
	7.0		5/5/8/10	8010			
	7.5	Grey, Green & Brown Silty Clay Firm, Moist, Plastic	7/10/12/15	8011			Oil In Gravel
	9.5	Stopped Hole 9.5					

TEST BORING RECORD

Driller Warren George, Inc. (G. Kutschera)

Project No. Hexcel Corporation, Lodi

Ground Water Elev. _____

Consultant _____

Date Started 6-2-84 Date Completed 6-2-84

Station No. _____

Size Casing _____ Total _____ ft.

Hole No. B9 Surface Elev. _____

Sample Size _____


Sheet 1 of 1 Sheets

Total No. Core Boxes _____

Sample Hammer Wt. _____ Lbs.; Drop _____ Ins

Size Core _____

Size Auger _____

ELEVATION	DEPTH	CLASSIFICATION OF MATERIAL	BLOWS & TIME	SAMPLE NO. & RUN	CORE RECOVERY	CORE LOSS	REMARKS
		Concrete & Gravel					Sample Not Take
	0.5	Brown Sand, Firm, Moist, Some Small Gravel	7/5/6/7	8020			
	2.5		12/14/14/12	8021			} Some 1 on water in samp ← Water Level
	4.5	 Grey Sand And Gravel, Firm, Wet	7/10/8/8	8022			
	6.5		7/4/5/4	8023			} Show oil In Sample
	8.0	Reddish Brown & Grey Silty Clay					
	8.5	Moist, Soft, Plastic	2/2/6/6	8024			
	10.5	Stopped Hole 10.5'					

TEST BORING RECORD

Driller Warren George, Inc. (G. Kutchera)

Project No. Hexcel Corporation, Lodi

Ground Water Elev. _____

Consultant _____

Date Started 6-2-84 Date Completed 6-2-84

Station No. _____

Size Casing _____ Total _____ ft.

Hole No. B10 Surface Elev. _____

Sample Size _____

Sheet 1 of 1 Sheets

Total No. Core Boxes _____

Sample Hammer Wt. 140# Lbs. Drop 30" Ins.


Size Core _____

Size Auger _____

ELEVATION	DEPTH	CLASSIFICATION OF MATERIAL	BLOWS & TIME	SAMPLE NO. & RUN	CORE RECOVERY	CORE LOSS	REMARKS
	0.5	Concrete					} Samples Not Taken.
	1.0	Gravel					
	3.0	Brown Sand Moist, Loose	7/6/5/4	8012			
	5.0		2/2/2/1	8013			4.4' Water Level
	7.0	Brown & Grey Sand & Gravel Wet, Loose	1/2/1/1	8014			
	7.5		2/5/5/5	8015			} Show of Clay water in sample
	8.0	Reddish Brown Silty Clay Loam					
	9.0	Moist, Firm, Plastic	4/5/5/5	8016			
	11.0	Stopped Hole 11.0'					

TEST BORING RECORD

Driller Warren George, Inc. (G. Kutcher) Project No. Hexcel Corporation, Lodi
 Ground Water Elev. _____ Consultant _____
 Date Started 6-2-84 Date Completed 6-2-84 Station No. _____
 Size Casing _____ Total _____ ft. Hole No. B11 Surface Elev. _____
 Sample Size _____ Sheet 1 of 1 Sheets
 Total No. Core Boxes _____ Sample Hammer Wt. 140 # Lbs., Drop 30" Ins
 Size Core _____ Size Auger _____

ELEVATION	DEPTH	CLASSIFICATION OF MATERIAL	BLOWS & TIME	SAMPLE NO. & RUN	CORE RECOVERY	CORE LOSS	REMARKS
	0.5	Concrete					Samples Not Taken
		Gravel & Cinders					
	1.0	Brown Sand Moist, Firm	3/6/3	Bag			
	2.5	Becomes Loose & wet below 2.5'	2/1/1/1	Bag			Slight Show of Oil on Water In Core,
	4.5	 Grey Sand & Gravel, Loose, wet	0/1/0/6	Bag			← <u>Water Level</u>
	6.5						} Oil In Bottom of Sampler } Oil In Gravel
	7.5	Reddish Brown & Grey Silty Clay Loam	4/4/3/5	Bag			
	8.5	Moist, Firm, Plastic	3/4/5/7	Bag			
	9.5	Stopped Hole 9.5'					



15 June 1984

analytical report

Lab. no. 56694-16

TENECH ENVIRONMENTAL ENGINEERS
744 W. Washington
South Bend, IN 46601

Attn: Mr. Charles Bishop

SAMPLE DESCRIPTION: "Hexcel"

date taken

date received 6/5/84

<u>Sample Description</u>	<u>Oil & Grease (Freon Solubles)</u> mg/kg
8000	1,250.
8001	3,880.
8002	3,610.
8003	4,000.
8004	149.
8009	7,950.
8010	6,070.
8011	650.
8013	10,100.
8014	3,890.
8015	2,280.
8016	174.

Results on a dry weight basis

Robert N. Bucaro
Robert N. Bucaro